

ADAMS HILL CENTER

Chase Subdivision, Lot 2A-1
Anchorage, AK

DRAINAGE REPORT

June 10, 2015



Completed By:

ANDERSON ENGINEERING
Michael E. Anderson, P.E.

Table of Contents

- 1. Statement of Compliance..... 2
- 2. Project Description 3
 - a. Location 3
 - b. Project Description..... 3
 - c. Category Determination..... 3
- 3. Basin Characterization..... 3
 - a. Pre-development Conditions..... 3
 - b. Post-development Conditions 3
 - c. Contributing Off-site Drainage..... 3
 - d. Floodways, Floodplains, and Wetlands 3
 - e. Problem Areas..... 4
 - f. Pre-development Runoff Analysis..... 4
- 4. Post-Development Runoff Analysis 4
- 5. Stormwater Conveyance Design 4
- 6. Stormwater Management Design 5
 - a. Design Criteria 5
 - a. Infiltration/Detention/Retention Facilities 5
- Appendix A: Drawings 6
- Appendix B: Drainage Calculations 7

DRAINAGE STATEMENT OF COMPLIANCE

| | | | |
|---|--|---|----------------|
| Applicant Name | | Contact Information (Phone and/or email) | |
| Mailing Address | 1118 E 70th Ave Suite 200 | | |
| Property Description (subdivision, lot(s), and block) | Chase Sub Lot 2A-1 | | |
| Plat Number | 060170 | MOA Assessor's Office Property Identification Number | 014-294-53-000 |
| MOA Tracking Number(s) (Indicate which provided) | C15 - | | |
| Project Category (Check one) | Crossing <input type="checkbox"/> Single-Lot Residential <input type="checkbox"/> Class 1A <input type="checkbox"/> Class 1B <input type="checkbox"/> Class 1C <input type="checkbox"/> Class 1D Small <input type="checkbox"/> Simple <input checked="" type="checkbox"/> Complex Large <input type="checkbox"/> Simple <input type="checkbox"/> Complex | | |
| Project and/or Report Title | Adams Hill Center | | |

By signature below, I certify that I am legally responsible for the drainage project described above and in the attached drainage report, and that it has been prepared in compliance with the MOA Project Management and Engineering Design Criteria Manual, the MOA Drainage Design Guidelines, the Anchorage Municipal Code, and other state and federal regulations and permits. I certify under penalty of law that the drainage report and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are penalties, including the possibility of fine and imprisonment, for knowing violations.

Signed

Date

2. Project Description

a. Location

This project is located in south Anchorage. The legal description is Chase Subdivision, Lot 2A-1. The project is located on the west side of Lake Otis Parkway, just south of East 88th Ave. **Latitude 61.1397°, Longitude 149.8351°.**

b. Project Description

The project includes the minor clearing of an empty lot and construction of a commercial building with a parking lot. Currently the lot is very flat, sloping slightly toward the north and west at less than a 1% slope, although it steepens just a bit at the northwest corner with flow to Arlon Street. Storm drain available in Lake Otis Parkway, but no storm drain currently fronts the project on Arlon Street. The subsurface soils on the lot are relatively silty gravels that have a percolation rate of approximately 20 min/in. The standard rainfall curves in this area of Anchorage are multiplied by a factor of 1.09.

c. Category Determination

This project is commercial in nature, involves less than five acres, and has only one building site. The estimated runoff for the 10-year, 24-hour storm after development is above the threshold limit of 0.42 CFS/acre, so this is classified as a **Complex Small Project**.

3. Basin Characterization

a. Pre-development Conditions

Currently, the property is a vacant lot with no real vegetation and an exposed gravel surface. The lot is relatively flat, sloping slightly toward the north and a bit to the west. The soils are silty gravel, and will absorb much of the water that falls onto the lot. Excess runoff flows to the north end of the lot where it sheds off either onto the north property or into the ROWs for Lake Otis Parkway and Arlon Street.

b. Post-development Conditions

After development, there will be a commercial building on the center of the lot surrounded by parking areas. The entire property will be developed with either parking or landscaped planting areas. Water from the parking area and roof will be directed toward the landscaped portions of the lot where it will be filtered by vegetation before it flows into catch basin manholes that connect to the storm drain or to an underground absorption trench.

c. Contributing Off-site Drainage

N/A

d. Floodways, Floodplains, and Wetlands

N/A

e. Problem Areas

N/A

f. Pre-development Runoff Analysis

The EPA SWMM 5.0 program was used to analyze the runoff using the pre-construction conditions. The output is included in Appendix B. The entire project site was modeled as one subcatchment 250' wide, sloping 1% to the north. The following table shows the basic program values used.

| | | | |
|---------------------------|---|--------------------------|-------------------------------|
| <u>Total area:</u> | <u>Soils:</u> | | |
| 1.00 acres | 'Silt Loam': K=6.69, ψ =0.26, Initial Deficit = default (0.25) | | |
| | <u>Pervious</u> | <u>Impervious</u> | <u>Zero-Impervious</u> |
| Description | Compacted gravel | Paved area | Roof |
| Area | 94.3% | 5.7% | 0.0% (of Impervious) |
| Manning's n | 0.024 | 0.011 | |
| Depression Storage | 0.05" | 0.05" | 0.05" |

According to this model, the total runoff for the 10-year storm is 1,152.4 ft³. The peak flow is 0.744 CFS. The total runoff for the 1-year storm is 407.6 ft³.

4. Post-Development Runoff Analysis

The EPA SWMM 5.0 program was used to analyze the runoff using the post-construction conditions. The output is included in Appendix B. The entire project site was modeled as one subcatchment 115' wide, sloping 1% to the south. The following table shows the basic program values used.

| | | | |
|---------------------------|---|--------------------------|-------------------------------|
| <u>Total area:</u> | <u>Soils:</u> | | |
| 1.00 acres | 'Silt Loam': K=6.69, ψ =0.26, Initial Deficit = default (0.25) | | |
| | <u>Pervious</u> | <u>Impervious</u> | <u>Zero-Impervious</u> |
| Description | Grass | Paved area | Roof |
| Area | 6.2% | 93.8% | 27.3% (of Impervious) |
| Manning's n | 0.24 | 0.011 | |
| Depression Storage | 0.05" | 0.05" | 0.05" |

According to this model, the peak post-development runoff for the 10-year, 24-hour storm is 1.30 CFS, and a total runoff of 4,851.9 ft³. The runoff for the 1-year storm is 2,316.6 ft³.

5. Stormwater Conveyance Design

The storm water conveyance system will be pavement swales and grassy channels that will channel the water to the storm drain system or to the subsurface absorption trench.

6. Stormwater Management Design

a. Design Criteria

1. Conveyance Design

The contributing area for this project is far less than 40 acres, so the 10-year, 24-hour storm is used for sizing the storm drain system. However, the site is small enough that the additional flow could not affect the municipal storm drain. There is no need for additional conveyance design.

2. Project Flood Bypass

In the event of the 100-year, 24-hour storm, the excess water will overflow into the ROWs for Lake Otis Parkway and Arlon Street. There is no threat to additional properties resulting from such a storm.

3. Wetland Retention

N/A

4. Water Quality Protection

All of the water will be collected in grassy swales before it is channeled into the storm drain system. The first 1/2" of runoff will be filtered by the vegetation.

5. Extended Detention

The excess post-development runoff from the 1-year, 24-hour storm must be detained for 12-24 hours.

6. Flood Hazard Protection

This criterion limits the post-development runoff into a storm drain to a 5% increase over the pre-development peak flow.

7. Downstream Impact Control

This criterion also limits the post-development runoff into a storm drain to a 5% increase over the pre-development peak flow. Therefore, this is taken care of in the same way as the flood hazard protection.

a. Infiltration/Detention/Retention Facilities

A 30' long x 3' wide x 4' effective depth absorption trench will be constructed at the northwest corner of the property. The runoff from the western portion of the parking area will be directed to a landscaped drainage swale along the western property line. Runoff water will filter through the landscaping and be absorbed into the ground. Excess runoff will be collected in a manhole in the northwest corner and directed to the absorption trench.

The trench has 330 square feet of absorption area including the sidewalls. With an absorption rate of 20 minutes per inch it will have the capacity to absorb 82.5 cubic feet of water per hour or 619 Gallons. Over a 24 hour period this equates to 1,980 cubic feet or nearly 14,850 gallons.

The vast majority of the runoff water will be directed to the front vegetated swale and the Municipal storm drain in Lake Otis Parkway. The roof drains will also be piped directly to the collection manhole prior to discharge.

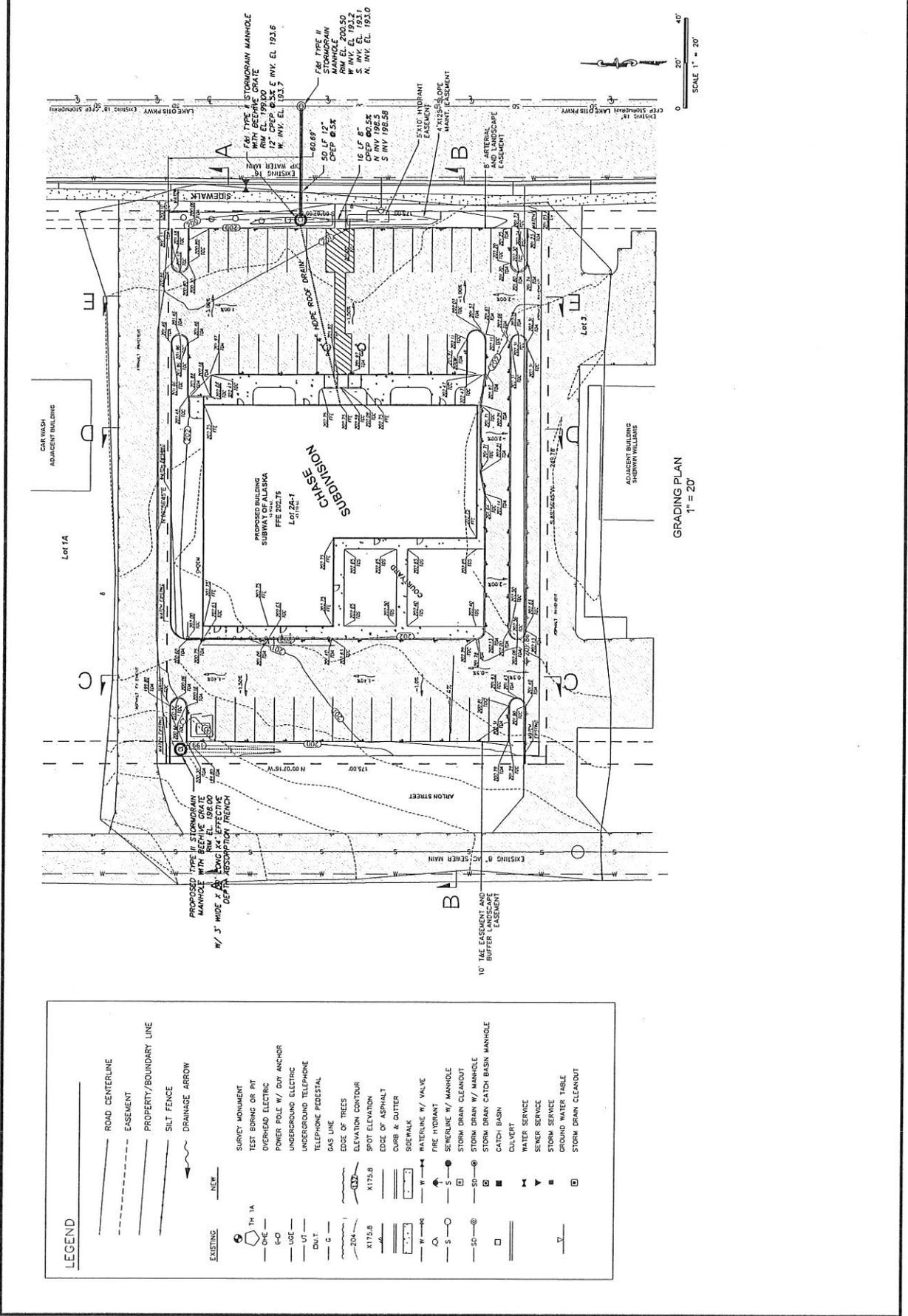
Appendix A: Drawings



ANDERSON ENGINEERING
 Anchorage, Alaska 99503
 1 907 577773
 800 5 CHASE
 1 907 473744

GRADING PLAN
SUBWAY OF ALASKA
 CHASE SUBDIVISION BLOCK 2, LOT 2A-1
 Anchorage, Alaska

| | | |
|--------------|-------------|----|
| DATE | DESCRIPTION | BY |
| 06/20/15 | | |
| SCALE | 100% | |
| GRADING PLAN | | |
| C3 | | |



SCALE 1" = 20'

GRADING PLAN
1" = 20'

| LEGEND | |
|--------|---------------------------------|
| — | ROAD CENTERLINE |
| - - - | EASEMENT |
| — | PROPERTY/BOUNDARY LINE |
| - - - | SILT FENCE |
| → | DRAINAGE ARROW |
| — | NEW |
| ⊙ | EXISTING |
| ⊙ | TH 1A SURVEY MONUMENT |
| ⊙ | TEST BOND OR PIT |
| ⊙ | OVERHEAD ELECTRIC |
| ⊙ | POWER POLE W/ GUY ANCHOR |
| ⊙ | UNDERGROUND ELECTRIC |
| ⊙ | UNDERGROUND TELEPHONE |
| ⊙ | TELEPHONE PEDISTAL |
| ⊙ | GAS LINE |
| ⊙ | EDGE OF TREES |
| ⊙ | ELEVATION CONTOUR |
| ⊙ | AT 75.8 SPOT ELEVATION |
| ⊙ | EDGE OF ASPHALT |
| ⊙ | CURB & GUTTER |
| ⊙ | SIDEWALK |
| ⊙ | WATERLINE W/ VALVE |
| ⊙ | FIRE HYDRANT |
| ⊙ | SEWERLINE W/ MANHOLE |
| ⊙ | STORM DRAIN CLEANOUT |
| ⊙ | STORM DRAIN W/ MANHOLE |
| ⊙ | STORM DRAIN CATCH BASIN MANHOLE |
| ⊙ | CATCH BASIN |
| ⊙ | CULVERT |
| ⊙ | WATER SERVICE |
| ⊙ | SEWER SERVICE |
| ⊙ | STORM SERVICE |
| ⊙ | GROUND WATER TABLE |
| ⊙ | STORM DRAIN CLEANOUT |



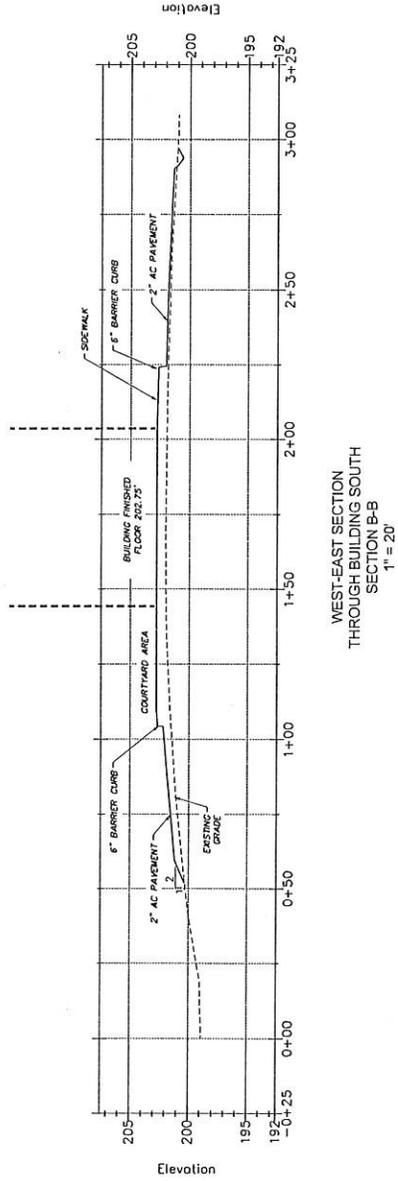
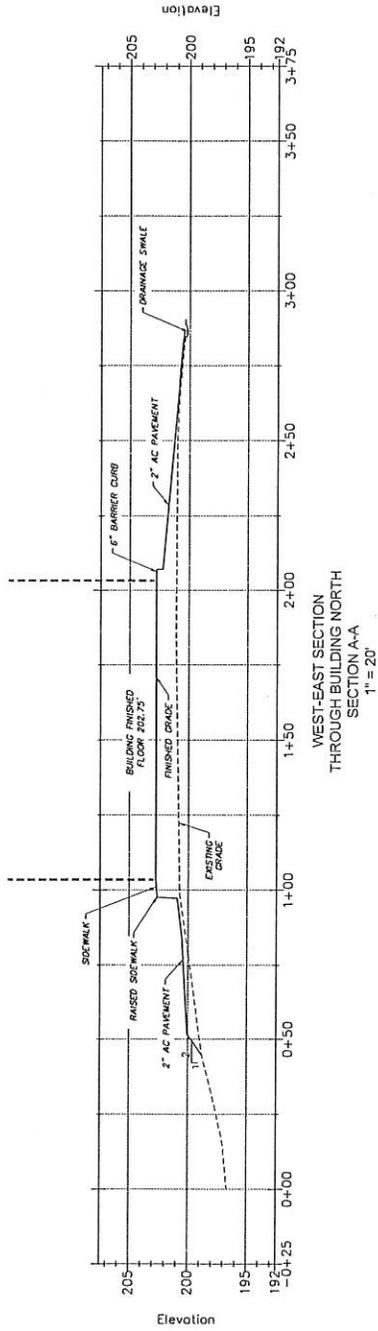
ANDERSON ENGINEERING
 1001 37773 Anchorage, Alaska 99518
 1001 37773

SITE SECTIONS 1
SUBWAY OF ALASKA
 CHASE SUBDIVISION BLOCK 2, LOT 2A-1
 Anchorage, Alaska

| | |
|----------|-------------------------------------|
| DATE | 06/2015 |
| SCALE | 100% |
| PROJECT | CHASE SUBDIVISION BLOCK 2, LOT 2A-1 |
| DESIGNER | ANDERSON ENGINEERING |
| CHECKED | |
| APPROVED | |
| DATE | 06/2015 |
| SCALE | 100% |
| PROJECT | CHASE SUBDIVISION BLOCK 2, LOT 2A-1 |
| DESIGNER | ANDERSON ENGINEERING |
| CHECKED | |
| APPROVED | |

SITE SECTIONS 1

C6



Appendix B: Drainage Calculations

**Section 1: Pre-Development Drainage Calculations
(EPA SWMM 5.0)**

PREDEVELOPMENT 1 YEAR 24 HOUR TOTAL FLOW

S U M M A R Y S T A T I S T I C S

=====

Object System
Variable Runoff (CFS)
Event Period Variable
Event Statistic Total (ft3)
Event Threshold Runoff > 0.0000 (CFS)
Event Threshold Event Volume > 0.0000 (ft3)
Event Threshold Separation Time >= 6.0 (hr)
Period of Record 04/20/2013 to 04/21/2013

Number of Events 1
Event Frequency* 0.888
Minimum Value 407.635
Maximum Value 407.635
Mean Value 407.635
Std. Deviation 0.000
Skewness Coeff. 0.000

*Fraction of all reporting periods belonging to an event.

PREDEVELOPMENT 10 YEAR 24 HOUR PEAK FLOW

S U M M A R Y S T A T I S T I C S

=====

Object System
Variable Runoff (CFS)
Event Period Variable
Event Statistic Peak (CFS)
Event Threshold Runoff > 0.0000 (CFS)
Event Threshold Event Volume > 0.0000 (ft3)
Event Threshold Separation Time >= 6.0 (hr)
Period of Record 04/20/2013 to 04/21/2013

Number of Events 1
Event Frequency* 0.925
Minimum Value 0.744
Maximum Value 0.744
Mean Value 0.744
Std. Deviation 0.000
Skewness Coeff. 0.000

*Fraction of all reporting periods belonging to an event.

PREDEVELOPMENT 10 YEAR 24 HOUR TOTAL FLOW

S U M M A R Y S T A T I S T I C S

=====

Object System
Variable Runoff (CFS)
Event Period Variable
Event Statistic Total (ft3)
Event Threshold Runoff > 0.0000 (CFS)
Event Threshold Event Volume > 0.0000 (ft3)
Event Threshold Separation Time >= 6.0 (hr)
Period of Record 04/20/2013 to 04/21/2013

Number of Events 1
Event Frequency* 0.925
Minimum Value 1152.384
Maximum Value 1152.384
Mean Value 1152.384
Std. Deviation 0.000
Skewness Coeff. 0.000

*Fraction of all reporting periods belonging to an event.

**Section 2: Post-Development Drainage Calculations
(EPA SWMM 5.0)**

POST DEVELOPMENT 1 YEAR 24 HOUR TOTAL

S U M M A R Y S T A T I S T I C S

=====

Object System
Variable Runoff (CFS)
Event Period Variable
Event Statistic Total (ft3)
Event Threshold Runoff > 0.0000 (CFS)
Event Threshold Event Volume > 0.0000 (ft3)
Event Threshold Separation Time >= 6.0 (hr)
Period of Record 04/20/2013 to 04/21/2013

Number of Events 1
Event Frequency* 0.579
Minimum Value 2316.617
Maximum Value 2316.617
Mean Value 2316.617
Std. Deviation 0.000
Skewness Coeff. 0.000

*Fraction of all reporting periods belonging to an event.

POST DEVELOPMENT 10 YEAR 24 HOUR PEAK FLOW

S U M M A R Y S T A T I S T I C S

=====

Object System
Variable Runoff (CFS)
Event Period Variable
Event Statistic Peak (CFS)
Event Threshold Runoff > 0.0000 (CFS)
Event Threshold Event Volume > 0.0000 (ft3)
Event Threshold Separation Time >= 6.0 (hr)
Period of Record 04/20/2013 to 04/21/2013

Number of Events 1
Event Frequency* 0.817
Minimum Value 1.300
Maximum Value 1.300
Mean Value 1.300
Std. Deviation 0.000
Skewness Coeff. 0.000

*Fraction of all reporting periods belonging to an event.

POST DEVELOPMENT 10 YEAR 24 HOUR TOTAL FLOW

S U M M A R Y S T A T I S T I C S

=====
Object System
Variable Runoff (CFS)
Event Period Variable
Event Statistic Total (ft3)
Event Threshold Runoff > 0.0000 (CFS)
Event Threshold Event Volume > 0.0000 (ft3)
Event Threshold Separation Time >= 6.0 (hr)
Period of Record 04/20/2013 to 04/21/2013

Number of Events 1
Event Frequency* 0.817
Minimum Value 4851.906
Maximum Value 4851.906
Mean Value 4851.906
Std. Deviation 0.000
Skewness Coeff. 0.000

*Fraction of all reporting periods belonging to an event.